Transportation Information

Central West Focus Area

Prepared for July 22, 2013 Steering Committee Meeting



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Intersection Spacing Distances

Question: What are the general spacing distances for intersections that are signalized, unsignalized, have roundabouts, or are restricted (for example, are right in/right out)?

Answer: On undivided roads, in general, spacing between signalized intersections should be a minimum of 1000 feet. Spacing for lower volume public streets and driveways should be 500-600 feet. Roundabouts would most likely have the same spacing, dependent upon the volumes of vehicles.

Right in/right out spacing distances are addressed on a case-by-case basis. Required spacing may be variable, depending on the circumstance.

Requirements for divided roads are more stringent. The following are links to NCDOT's current policies regarding divided roads:

- Median Crossover Guidelines: https://connect.ncdot.gov/resources/safety/Congestion%20Mngmt%20and%20Signing/Congestion%20Mngmt%20Carolina%20Median%20Crossover%20Guidelines%20Current%20Edition%202004-01-01.pdf
- Facility Type and Control of Access
 Definitions: https://connect.ncdot.gov/projects/planning/TPB%20%20Strategic%20Highway%20
 Corridors/NCDOT%20Facility%20Types%20-%20Control%20of%20Access%20Definitions.pdf

Note: All these issues should be evaluated on an individual basis.

Step 1 Generate Total Trips for Study Area

Trip Production

of Units in area

Single Family
Multi Family
Senior Housing

Trip Attraction

sq. feet in area

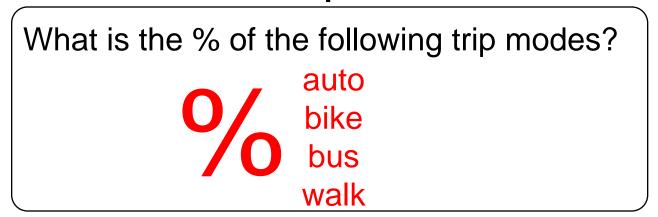
retail office medical office X Trip Production Trips

X Trip Attraction Trips

Total Trips

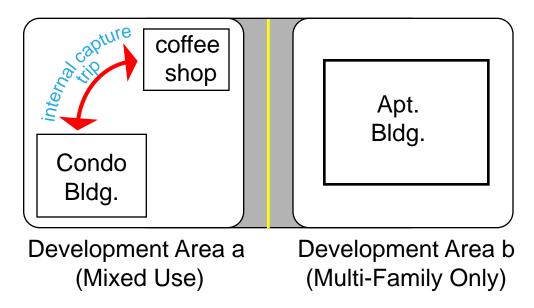
Step 2 Make Assumptions about Trips

Assumption 1: Mode Splits (applies to production and attraction trips)

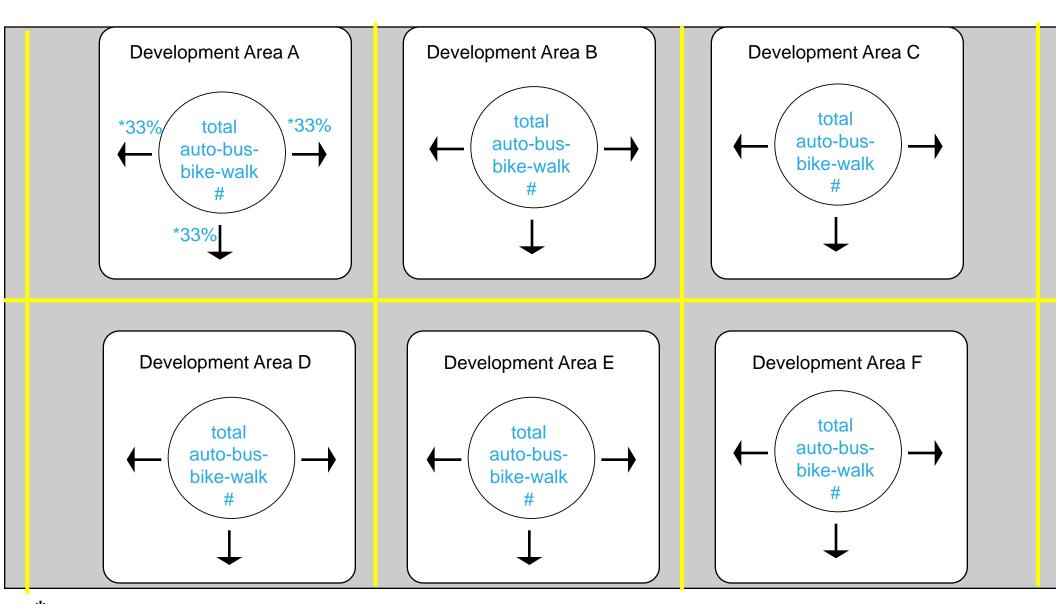


Assumption 2: Internal Trip Capture

What percentage of trips are likely to stay within a development area?



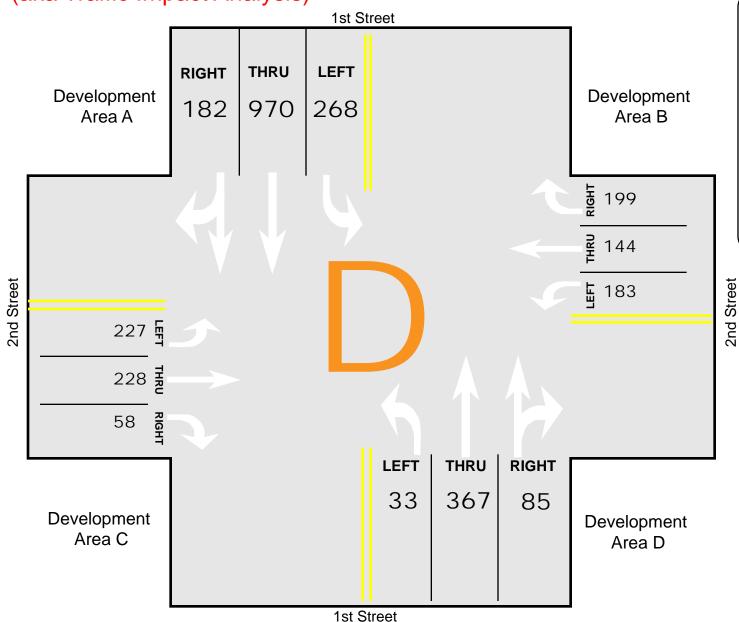
Step 3 "Assign" all of those trips to the road network.



^{*} the percentages used to assign trips in this example are for illustration purposes only. The percentages may vary but for a development area they must add up to 100%.

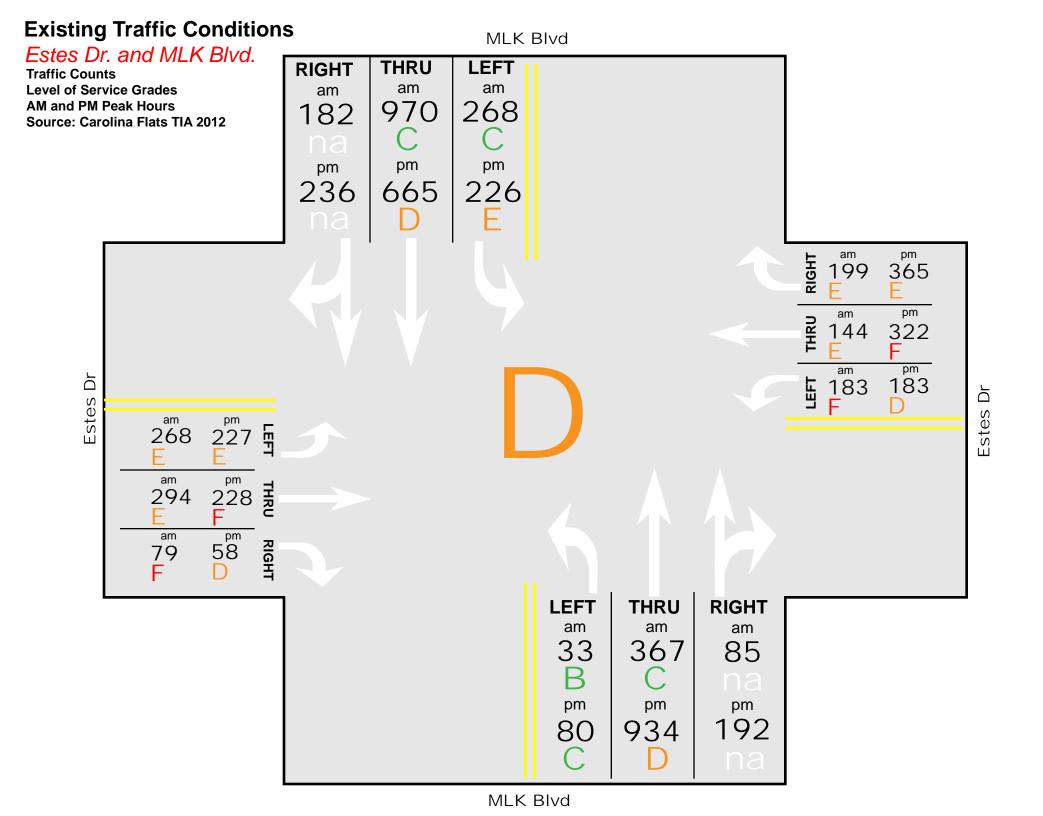
Step 4 - After Trips are assigned, use Engineering software to model the impacts on the network and intersections.

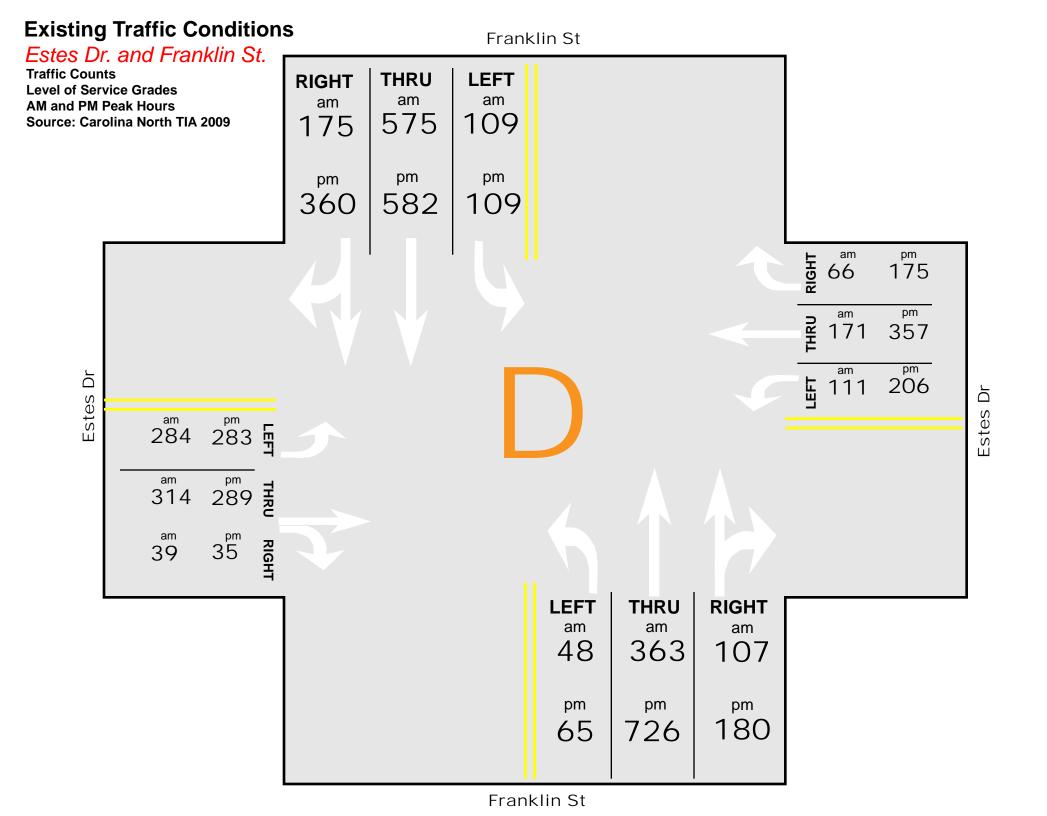
(aka Traffic Impact Analysis)

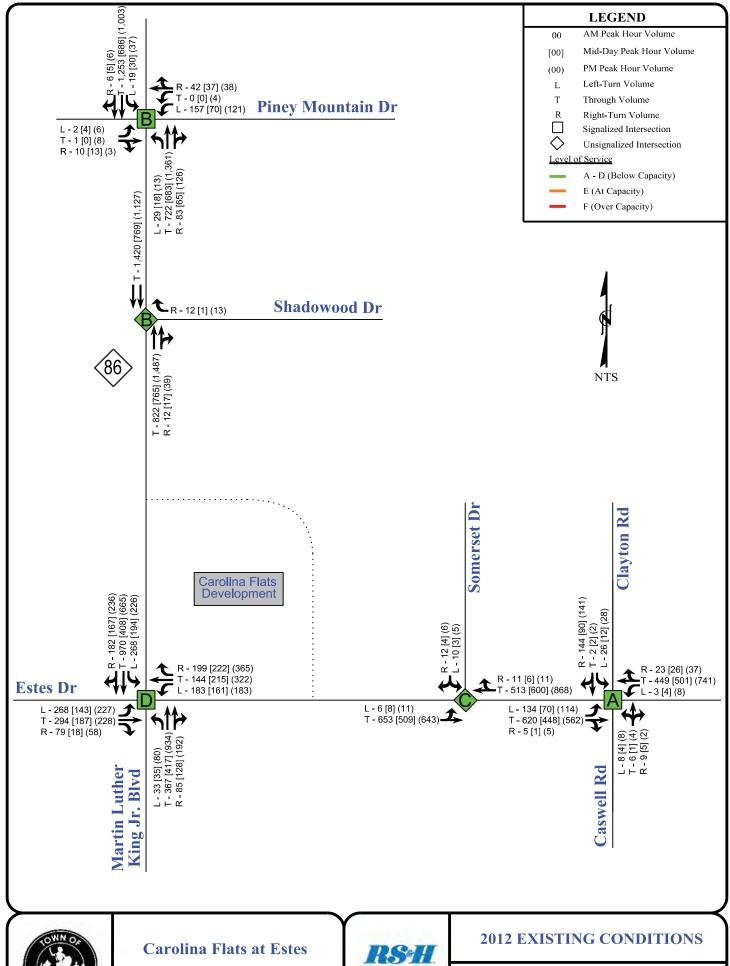


For this example, 2nd Street in the west bound direction during rush hour(aka. peak) had 199 cars turning right, 144 cars going straight, and 183 cars turning left.

The intersection's overall level of service was scored as a D. Level of Service or LOS is a traffic impact metric that uses expected delay and the total volume of trips on a particular roadway.









Traffic Impact Analysis



Table 2: Intersection Capacity Analysis - 2012 Existing Traffic Conditions

| | Traffic Movement | | Level of Service | | | Volume-to-Capacity Ratio | | |
|--|----------------------|-----|------------------|---------------------|------------|-----------------------------|---------------------|------------|
| Intersection | | | AM Peak | Mid- day Peak | PM Peak | AM Peak | Mid- day Peak | PM Peak |
| Martin Luther King, Jr. Boulevard at Piney Mountain Road | Overall Intersection | | В | В | В | N/A | N/A | N/A |
| | Eastbound | L | F | F | F | .05 | .08 | .13 |
| | | TR | F | F | F | .25 | .28 | .23 |
| | Westbound | L | F | F | F | .68 | .42 | .62 |
| | | TR | F | F | F | .40 | .49 | .47 |
| | Northbound | L | Α | Α | Α | .12 | .03 | .04 |
| | | TR | Α | Α | Α | .32 | .29 | .58 |
| | Southbound | L | Α | Α | Α | .04 | .06 | .21 |
| | | TR | Α | Α | Α | .46 | .25 | .37 |
| Martin Luther King, Jr. Blvd at Shadowood Dr | Westbound | R | В | А | А | .02 | .01 | .02 |
| Martin Luther King, Jr. Boulevard at Estes Drive | Overall Intersection | | D | D | D | N/A | N/A | N/A |
| | Eastbound | L | Е | Е | F | .82 | .79 | .91 |
| | | Т | F | F | E | .88 | .79 | .60 |
| | | R | D | D | D | .23 | .06 | .15 |
| | Westbound | L | F | Е | D | .96 | .77 | .62 |
| | | Т | Е | F | F | .58 | .85 | .98 |
| | | R | Е | Е | E | .57 | .72 | .86 |
| | Northbound | L | В | В | С | .17 | .07 | .31 |
| | | TR | С | С | D | .27 | .29 | .76 |
| | Southbound | L | С | В | Е | .51 | .39 | .91 |
| | | TR | D | В | С | .65 | .30 | .57 |
| Estes Drive at Somerset Drive | Eastbound | TL | Α | А | Α | .01 | .01 | .02 |
| | Southbound | LR | С | С | D | .09 | .03 | .08 |
| Estes Drive at Caswell Road/Clayton Road | Overall Intersection | | Α | Α | Α | N/A | N/A | N/A |
| | Eastbound | L | Α | Α | Α | .42 | .12 | .31 |
| | | TR | Α | Α | Α | .32 | .34 | .43 |
| | Westbound | L | Α | Α | Α | .29 | .01 | .02 |
| | | TR | Α | Α | Α | .29 | .40 | .59 |
| | Northbound | LTR | С | С | С | .14 | .07 | .10 |
| | Southbound | L | С | С | С | .20 | .09 | .22 |
| | | TR | В | В | В | .53 | .41 | .41 |

Table 2: N/A - Not Applicable



